

# How to treat the coupling issue of the Saint-Venant-Exner system of equations

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joint work with  
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June 1<sup>st</sup>, 2015

# Outline

Context & Motivations

Numerical scheme

Test cases

Discussion

## Context & Motivations

# Motivations

## Framework

Sediments transport is responsible of modification of river beds.

2 processes of sediments transport:

- by suspension: particles can be found on the whole vertical water depth and rarely be in contact with the bed,
- by bedload: particles are moving near the bed by saltation and rolling.

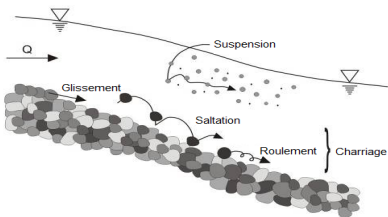


Figure: Processes of sediment transport.

Thereafter, we only focus on the **bedload transport**.

# Saint-Venant–Exner equations

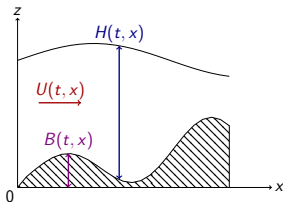
## The model

In the literature, most of industrial codes use the **Saint-Venant–Exner model**.

$$\begin{cases} \partial_t H + \partial_x (Q) = 0, & (1a) \\ \partial_t Q + \partial_x \left( \frac{Q^2}{H} + \frac{gH^2}{2} \right) = -gH\partial_x B - \frac{\tau}{\rho}. & (1b) \\ \partial_t B + \partial_x Q_s = 0, & (1c) \end{cases}$$

Coupled model between:

- the Saint-Venant equations (aka shallow-water equations): (1a)–(1b)



$H(t, x)$ : water height,  
 $Q(t, x) = HU$ : discharge,  
 $B(t, x)$ : bottom topography,  
 with  $x \in \Omega \subseteq \mathbb{R}$ ,  $t \geq 0$ .

# Saint-Venant–Exner equations

## The model

$\tau$  is defined by the **Manning formula**,

$$\tau = \rho g H \frac{Q |Q|}{H^2 K_s^2 R_h^{4/3}}, \quad (2)$$

where, in the particular case of a rectangular channel with width  $l$ , the hydraulic radius  $R_h$  reads

$$R_h = \frac{lH}{l + 2H}.$$

# Saint-Venant–Exner equations

## The model

- the Exner equation (1c)

where  $Q_s(t, x)$  is the solid transport flux defined by

$$Q_s = \sqrt{\frac{g(\rho_s - \rho)d^3}{\rho}} Q_s^*(\tau^*; \tau_c^*) \frac{\tau^*}{|\tau^*|} \quad (3)$$

and the **Meyer-Peter-Müller formula**,

$$Q_s^* = A (|\tau^*| - \tau_c^*)_+^{3/2} \quad (4)$$

with

{	$A$	a constant,
	$\rho_s, \rho$	resp. the mass densities of the solid and fluid phases,
	$g$	the gravitational acceleration,
	$\tau^*$	the shear stress (aka Shields parameter),
	$\tau_c^*$	the critical value for the initiation of motion,
	$d$	the grain diameter.

# Saint-Venant–Exner equations

## The model

A more practical expression of the solid discharge

- Grass formula,

$$Q_s = A_g U |U|^{m-1} \quad (5)$$

where  $A_g$  is an empirically determined constant and  $0 < m < 4$ .



# Saint-Venant–Exner equations

## The model

The Saint-Venant–Exner equations can be rewritten in a vectorial form,

$$\partial_t \tilde{W} + \partial_x F(\tilde{W}) = S(\tilde{W}), \quad (6)$$

where

$$\tilde{W} = \begin{pmatrix} H \\ HU \\ B \end{pmatrix}, \quad F(\tilde{W}) = \begin{pmatrix} HU \\ HU^2 + \frac{gH^2}{2} \\ Q_s \end{pmatrix},$$

$$S(\tilde{W}) = \begin{pmatrix} 0 \\ -gH\partial_x B \\ 0 \end{pmatrix}.$$

**Quasilinear form:**

$$\partial_t \tilde{W} + A(\tilde{W})\partial_x \tilde{W} = S(\tilde{W}),$$

where  $A$  is the jacobian matrix of  $F$ .

# Motivations

## Numerical aspect

Two strategies to approximate the solution of the system:

**splitting and non-splitting methods.**

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## Numerical aspect

Two strategies to approximate the solution of the system:

### splitting and non-splitting methods.

The problem of choice between these two methods remains when considering “fast flow” (Hudson et al, 2003 & 2005):

- the splitting method injects numerical instabilities,

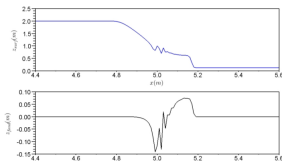


Figure: Free surface (top) and Bottom topography (bottom)

# Motivations

## Numerical aspect

- the non-splitting method allows to correct these instabilities,
  - Roe-type solver (Hudson et al. 2003 & 2005, Murillo and Garcia-Navarro 2010),
  - Intermediate Field Capturing Riemann solver (Pares 2006, Pares et al. 2011),
  - Relaxation scheme (Delis et al. 2008, ABCDGJSGS 2011),
  - Non Homogeneous Riemann solver (Benkhaldoun et al. 2009),
  - Godunov-type method based on a three-waves Approximate Riemann Solver (ARS).

## Numerical scheme

# Numerical approximation

## Properties & Main definitions

- **Positivity** of water height,

$$H \geq 0,$$

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- **Well-balanced** property or ability to preserve steady states of the lake at rest,

$$U = 0, \quad H + B = \text{Cte}.$$



# Numerical approximation

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$$U = 0, \quad H + B = \text{Cte}.$$

- **Froude number**,

$$F_r = \frac{|U|}{\sqrt{gH}}. \quad (7)$$

- $F_r < 1$  Fluvial regime,
- $F_r = 1$  Transcritical regime,
- $F_r > 1$  Torrential regime.

# Numerical scheme

## Objective

### Main objective

Developing a non-splitted method to solve the Saint-Venant–Exner system.

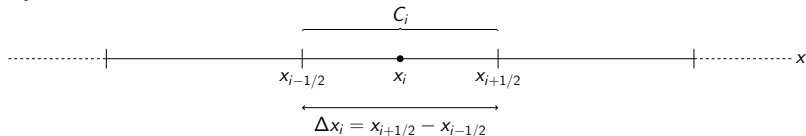
### Strategy

Propose a Godunov-type method to solve the Saint-Venant–Exner equations based on the design of a **three-wave Approximate Riemann Solver** which is able to degenerate to an ARS satisfying all these properties together when the solid flux is null, sufficiently **easy to compute**.

# Numerical scheme

## Discretization

**Space discretization**  $\Omega, \forall i \in \mathbb{Z}$



$N_x$ : Number of cells.

**Time discretization**  $t \geq 0, \forall n \in \mathbb{N}$

$$t^{n+1} = t^n + \Delta t^n, \quad \Delta t > 0.$$

In the following, we denote

$$\Delta x_i = \Delta x, \quad \Delta t^n = \Delta t.$$

# Numerical scheme

## Main ideas

Notations:  $\forall X \in \{H, HU, B\}$ ,

$$X_L \approx \frac{1}{\Delta x} \int_{-\Delta x}^0 X(x) dx; X_R \approx \frac{1}{\Delta x} \int_0^{\Delta x} X(x) dx; X_i \approx \frac{1}{\Delta x} \int_{C_i} X(x) dx.$$

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At  $t^n$ ,  $\tilde{W}_i^n = (W_i^n, B_i^n) = (H_i^n, H_i^n U_i^n, B_i^n)^T$  a given piecewise constant approximate solution,

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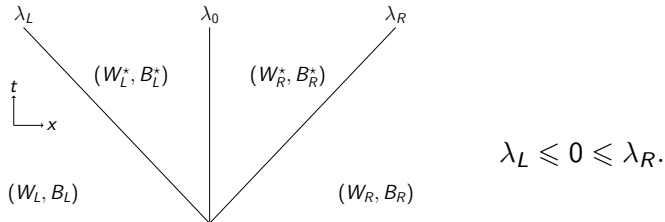
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- Building an approximate solution of the Riemann problem at each interface  $x_{i+1/2}$ ,
- Definition of  $\tilde{W}_i^{n+1} = (W_i^{n+1}, B_i^{n+1})$  by calculating the average value of the juxtaposition of these solutions in each cell  $C_i$  at time  $t^{n+1}$ .

# Numerical scheme

## Riemann problem

A simple Approximate Riemann Solver composed by three waves propagating with velocities  $\lambda_L$ ,  $\lambda_0 = 0$  and  $\lambda_R$  such as



**Figure:** Local Riemann problem

gives an approximate Riemann solution associated with initial data

$$(W(0, x), B(0, x)) = \begin{cases} (W_L, B_L) & , x < 0, \\ (W_R, B_R) & , x > 0. \end{cases}$$

**CFL condition:**  $\Delta t < \frac{\Delta x}{2 \max(|\lambda_L|, \lambda_R)}.$



# Numerical scheme

## Consistency

### Consistency

$$F(\tilde{W}_R) - F(\tilde{W}_L) - S(\tilde{W}_L, \tilde{W}_R) = \lambda_L(\tilde{W}_L^* - \tilde{W}_L) + \lambda_R(\tilde{W}_R - \tilde{W}_R^*),$$

with

$$\lim_{\substack{\tilde{W}_L, \tilde{W}_R \rightarrow \tilde{W} \\ \Delta x \rightarrow 0}} \frac{1}{\Delta x} S(\tilde{W}_L, \tilde{W}_R) = (0, -gH\partial_x B, 0)^T.$$

### Relations of consistency in the integral form:

$$\left\{ \begin{array}{l} H_R U_R - H_L U_L = \lambda_L(H_L^* - H_L) + \lambda_R(H_R - H_R^*), \quad (8) \\ \left( H_R U_R^2 + \frac{gH_R^2}{2} \right) - \left( H_L U_L^2 + \frac{gH_L^2}{2} \right) + g\Delta x \{H\partial_x B\} \\ \quad = \lambda_L(H_L^* U_L^* - H_L U_L) + \lambda_R(H_R U_R - H_R^* U_R^*), \quad (9) \\ Q_{sR} - Q_{sL} = \lambda_L(B_L^* - B_L) + \lambda_R(B_R - B_R^*). \quad (10) \end{array} \right.$$

# Numerical approximation of the Saint-Venant–Exner equations

Definition of the intermediate states

Relations of continuity across the stationary wave:

$$\left\{ \begin{array}{l} H_L^* + B_L^* = H_R^* + B_R^*, \\ H_L^* U_L^* = H_R^* U_R^*. \end{array} \right. \quad (11)$$

$$\left\{ \begin{array}{l} H_L^* U_L^* = H_R^* U_R^*. \end{array} \right. \quad (12)$$

We add a minimization problem

$$\min F(B_L^*, B_R^*) = (\|B_L - B_L^*\|^2 + \|B_R - B_R^*\|^2)$$

$$u.c. \lambda_L (B_L^* - B_L) + \lambda_R (B_R - B_R^*) - (Q_{sR} - Q_{sL}) = 0$$

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$$B_L^* = B_L + \frac{\lambda_L}{\lambda_L^2 + \lambda_R^2} \Delta Q_s, \quad (13)$$

$$B_R^* = B_R - \frac{\lambda_R}{\lambda_L^2 + \lambda_R^2} \Delta Q_s, \quad (14)$$

# Numerical scheme

Definition of the intermediate states: Well-balanced property

$$Q^* := H_L^* U_L^* = H_R^* U_R^*,$$

$$Q^* = Q_{HLL} - \frac{g}{\lambda_R - \lambda_L} \Delta x \{H \partial_x B\}, \quad (15)$$

with

$$Q_{HLL} = \frac{\lambda_R H_R U_R - \lambda_L H_L U_L}{\lambda_R - \lambda_L} - \frac{\left( H_R U_R^2 + \frac{g H_R^2}{2} \right) - \left( H_L U_L^2 + \frac{g H_L^2}{2} \right)}{\lambda_R - \lambda_L},$$

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Well-balanced property ensures by

$$\{H \partial_x B\} = \begin{cases} \frac{H_L + H_R}{2 \Delta x} \min(H_L, \Delta B) & \text{if } \Delta B^* \geq 0, \\ \frac{H_L + H_R}{2 \Delta x} \max(-H_R, \Delta B) & \text{if } \Delta B^* < 0. \end{cases}$$

# Numerical approximation of the Saint-Venant–Exner equations

Definition of the intermediate states: Positivity of the water height

$$H_L^* = H_{HLL} + \frac{\lambda_R}{\lambda_R - \lambda_L} \Delta B^*, \quad (17)$$

$$H_R^* = H_{HLL} + \frac{\lambda_L}{\lambda_R - \lambda_L} \Delta B^*, \quad (18)$$

with

$$H_{HLL} = \frac{\lambda_R H_R - \lambda_L H_L}{\lambda_R - \lambda_L} - \frac{1}{\lambda_R - \lambda_L} (H_R U_R - H_L U_L). \quad (19)$$

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If $\Delta B^* \geq 0$ ,	If $\Delta B^* < 0$ ,
$\lambda_R \tilde{H}_R^* = \max(\lambda_R H_R^*, 0)$ ,	$\lambda_L \tilde{H}_L^* = \max(\lambda_L H_L^*, 0)$
$\lambda_L \tilde{H}_L^* = \lambda_L H_L^* - \lambda_R (H_R^* - \tilde{H}_R^*)$ ,	$\lambda_R \tilde{H}_R^* = \lambda_R H_R^* - \lambda_L (H_L^* - \tilde{H}_L^*)$

# Numerical approximation of the Saint-Venant–Exner equations

## Definition of the wave velocities

The main issue comes from the **choice of  $\lambda_L$  and  $\lambda_R$** .



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Recall

$$A(\tilde{W}) = \begin{bmatrix} 0 & 1 & 0 \\ gH - U^2 & 2U & gH \\ \tilde{\alpha} & \tilde{\beta} & 0 \end{bmatrix}$$

where  $\tilde{\alpha} = \frac{\partial Q_s}{\partial H}$  and  $\tilde{\beta} = \frac{\partial Q_s}{\partial Q}$ .

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where  $\tilde{\alpha} = \frac{\partial Q_s}{\partial H}$  and  $\tilde{\beta} = \frac{\partial Q_s}{\partial Q}$ .

**Characteristic polynomial of  $A$ :**

$$p_A(\lambda) = \lambda^3 - 2U\lambda^2 - (gH(1 + \tilde{\beta}) - U^2)\lambda - gH\tilde{\alpha} = 0. \quad (20)$$

# Numerical approximation of the Saint-Venant–Exner equations

Definition of the wave velocities: Nickalls' bounds (2011)

**Derivative quadratic equation of  $p_A$ :**

$$3\lambda^2 - 4U\lambda - (gH(1 + \tilde{\beta}) - U^2) = 0. \quad (21)$$

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The solutions are

$$\lambda_{\pm} = x_0 \pm \Omega, \quad (22)$$

such as  $x_0 = \frac{2U}{3}$  and  $\Omega = \frac{1}{3}\sqrt{U^2 + 3gH(1 + \tilde{\beta})}$ .

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The wave velocities are defined by

$$\lambda_L = x_0 - 2\Omega, \quad (23)$$

$$\lambda_R = x_0 + 2\Omega. \quad (24)$$

# Numerical scheme

## Summary

### Associated Godunov-type scheme

$$\begin{cases} \tilde{W}_i^{n+1} = \tilde{W}_i^n - \frac{\Delta t^n}{\Delta x} (F_{i+1/2}^- - F_{i-1/2}^+), \\ \tilde{W}_i^0 = \frac{1}{\Delta x} \left( \int_{C_i} H_0(x) dx, \int_{C_i} (H_0 U_0)(x) dx, \int_{C_i} B_0(x) dx \right)^T, \end{cases}$$

where  $F^-$  and  $F^+$  are given by

$$\begin{cases} F^-(\tilde{W}_L, \tilde{W}_R) = F(\tilde{W}_L) + \lambda_L (\tilde{W}_L^* - \tilde{W}_L), \\ F^+(\tilde{W}_L, \tilde{W}_R) = F(\tilde{W}_R) + \lambda_R (\tilde{W}_R^* - \tilde{W}_R), \end{cases}$$

and the wave velocities are defined by

$$\lambda_L = x_0 - 2\Omega,$$

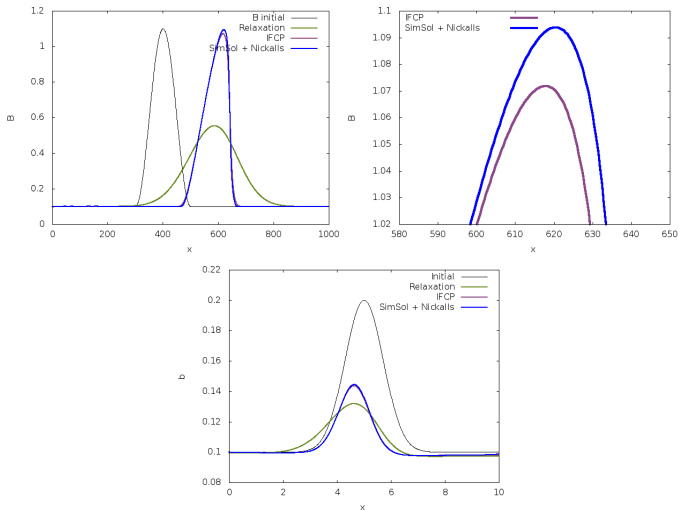
$$\lambda_R = x_0 + 2\Omega,$$

with  $x_0 = \frac{2U}{3}$  and  $\Omega = \frac{1}{3} \sqrt{U^2 + 3gH(1 + \tilde{\beta})}$  and  $\tilde{\beta} = \frac{\partial Q_s}{\partial Q}$ .

## Test cases

# Numerical results

## Evolution of a bump in a fluvial regime



**Figure:** Bump in fluvial (top) and transcritical (bottom) flow.



# Numerical results

## Evolution of a bump in a torrential regime

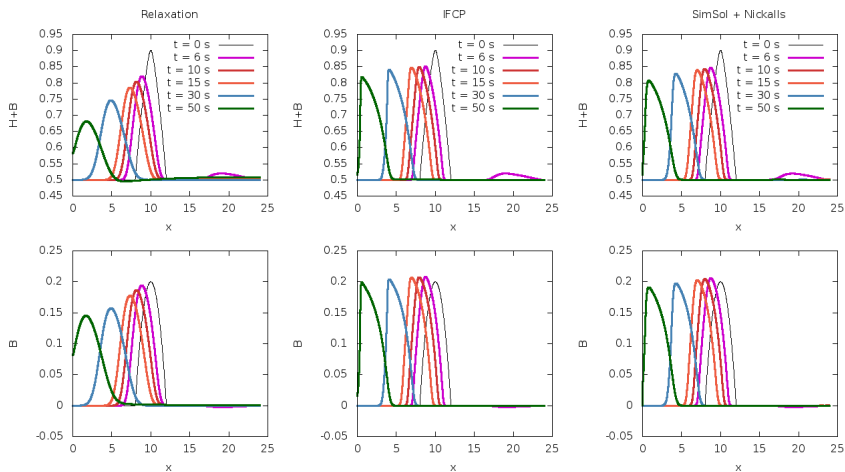
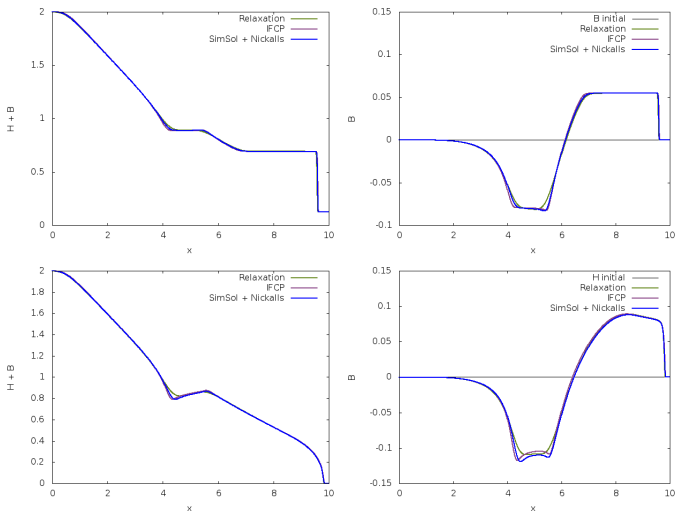


Figure: Antidune.

# Numerical results

## Dam break over a wet bed

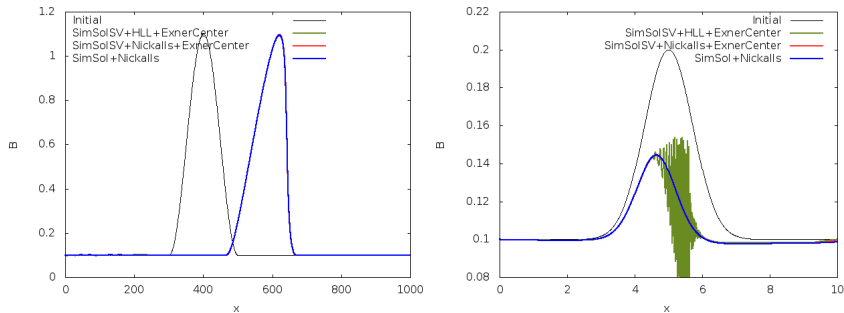


**Figure:** Dam break over wet (top) and dry (bottom) topographies.

# Discussion

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## Splitting vs non-splitting methods



**Figure:** Bump on fluvial (left) and transcritical (right) regimes.

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## Splitting vs non-splitting methods

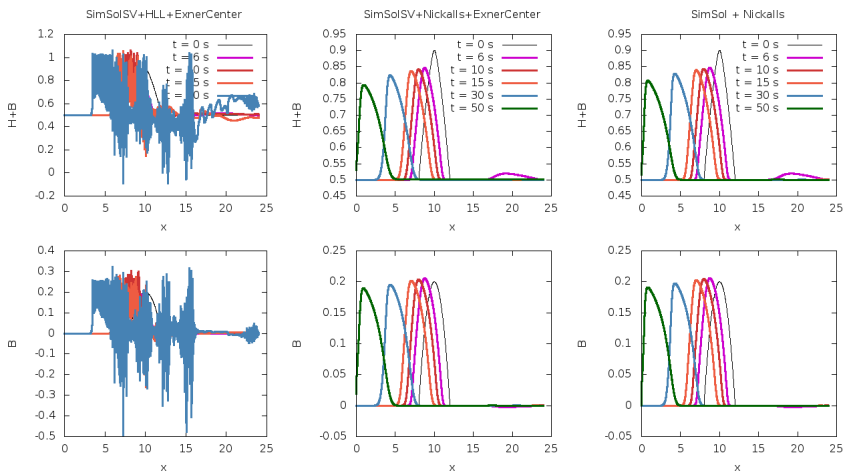


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## Splitting vs non-splitting methods

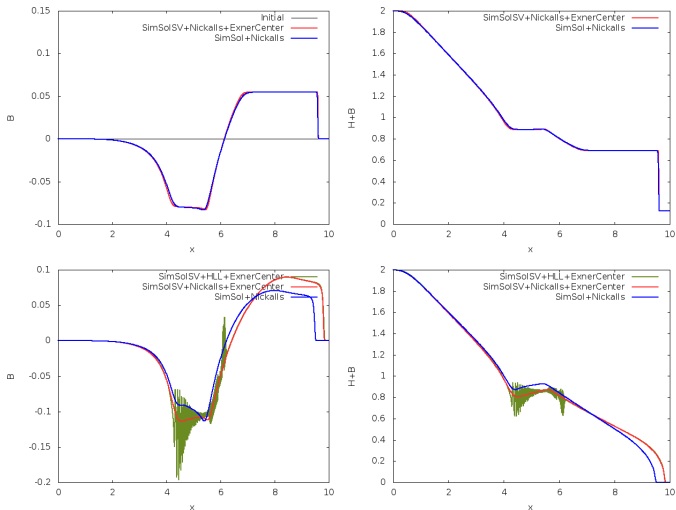


Figure: Dam break over wet (top) and dry (bottom) bed.

Thank you for your attention!